## **Design Optimization of Bay Spacing of Steel Foot Over Bridge**

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**Abstract**— Bridge is a structure that offers passage over limitations/obstacles which includes valleys, tough terrain or river or bodies of water via spanning such obstacles with herbal or manmade substances/material. The smooth kind of a bridge is steppingstones, so this may were one of the premature sorts of a foot bridge. Neolithic people also construct a shape of a boardwalk to the other facet of swamp, of which the sweet track, and the post track are examples from England, which are about 6000 years antique. Doubtless past peoples might also have used Timber Bridge that drop naturally. Different kinds of design foot over bridges. Steel truss is normally used round the world for the development of foot over bridges of different length. steel is a useful material that provides provable solution. Steel has long been diagnosed as the financial/economical option for a number of foot over bridges.

In the present study, design of economical section using different bay spacing is studied. For this purpose, analysis of foot over bridge has been carried out by using staad-pro software. The comparison is made based on weight of structure, utility ratio, Deflection. It is found about 50% weight of the structure is reduced after optimization.

Index Terms - Foot Over Bridge, Utility Ratio, Deflection.

#### Introduction

A foot over bridge is a bridge designed only for pedestrian. While the principle which means for a bridge is a structure which links "two distinct points at a peak above the earth". The smooth kind of a bridge is steppingstones, so this may were one of the premature sorts of a foot bridge. Neolithic people also construct a shape of a boardwalk to the other facet of swamp, of which the sweet track, and the post track are examples from England, which are about 6000 years antique Doubtless past peoples might also have used Timber Bridge that drop naturally. Different kinds of design foot over bridges. Steel truss is normally used round the world for the development of foot over bridges of different length. steel is a useful material that provides provable solution. Steel has long been diagnosed as the financial/economical option for a number of foot over bridges.

After literature survey it is observed that, includes design and analysis of Foot over Bridge Using STAAD-PRO software, study mainly includes static and dynamic seismic analysis, specifically seismic coefficient method and response spectrum method is used But, yet bay spacing for economical design of steel foot over bridge has not been studied. So, in the present dissertation work, it is proposed to carry out bay spacing for the optimize/economical design of steel foot over bridge.

## **Literature Review**

A bridge is a structure built for carrying the road/railway traffic or other moving loads over a depression or gap or obstacle such as river, channel, canyon, valley, road or railway. Depending on the purpose and the obstacle the type of bridge is selected to meet the requirement if a bridge is constructed to carry a highway traffic it is called highway bridge if it used to carry a railway traffic it is called as railway bridge the bridges that are constructed exclusively to carry pedestrians, cycle and animals are known as foot bridges and bridges used to carry canals and pipe lines are known as aqueduct bridge

#### **Necessity of Foot Over Bridge**

In recent trends there are various upcoming projects of foot over bridge in various sectors such as industrial or infrastructure. So, we have to design a economical section using bay spacing and how to change utility ratio with span increases and bay spacing changes. We have to see the changes in utility ratio with built-up sections for different span variations of bay spacing and adopt economical section. Therefore, in the present dissertation work, it is proposed to carry out bay spacing for economical/optimize design of steel foot over bridge.

#### Modelling and analysis

To obtain and compare the results for different bay spacing different types of model designed and analysis ids carried out by using staad-pro software.

# A. Problem Data For design Of Foot Over Bridge

| 0                   |                           |  |  |  |
|---------------------|---------------------------|--|--|--|
| Type of Structure:  | Steel                     |  |  |  |
| Seismic zone:       | III                       |  |  |  |
| Resonance Reduction | 3                         |  |  |  |
| Factor:             |                           |  |  |  |
| Size of span & bay  | 10m Span & 2m bay spacing |  |  |  |
| spacing:            | 10m Span & 3m bay spacing |  |  |  |
|                     | 10m Span & 4m bay spacing |  |  |  |
|                     | 10m Span & 5m bay spacing |  |  |  |
|                     | 15m Span & 2m bay spacing |  |  |  |
|                     | 15m Span & 3m bay spacing |  |  |  |
|                     | 15m Span & 4m bay spacing |  |  |  |
|                     | 15m Span & 5m bay spacing |  |  |  |
| Importance Factor:  | 1.5                       |  |  |  |
| Soil Type:          | Hard                      |  |  |  |
| Damping Ratio:      | 0.02                      |  |  |  |
| 3143                |                           |  |  |  |

| Self-weight floor load: | $-2KN/m^2$            |
|-------------------------|-----------------------|
| Pressure on floor:      | $-5KN/m^2$            |
| Wind Load Intensity:    | 0.18kN/m <sup>2</sup> |
| Height:                 | 4 m                   |

## B. Step By Step Procedure of Analysis In STAAD-PRO V8i. Define Material Properties:

The material properties of concrete and steel are given. for concrete properties like weight perunit volume, modulus of elasticity, poisons ratio must be given. For steel minimum yield strength is required and for Fe415, it is 415000kN/m<sup>2</sup>

## **Define Section Properties**

The frames section properties of all truss member size, thickness size is given. Bay spacing for different span arrangements are given.

## Develop The Model And Assign The Joint Restraints:

In this step, preparing model by adding frame objects with the associated properties. Once the model is prepared next step is assigning joint restraints. For building frame, the joint restraints one end is fixed and another end is fixed but.

## Develop Load Pattern And Assign To Frame:

In this step, the load patterns like dead load, dead wall, floor finish, live load are defined. Theloading is given to the frame. The mass source is defined the all dead load are considered 100%. Then all joints are making rigid by joint constraints. The loading combinations are given.

## Run The Analysis :

The value for utility ratio, weight of structure and deflection for different bay spacing must be check.

## Verify All Members Are Passed:

Verify every element of structure is passed through check.

Design the section for optimum design:

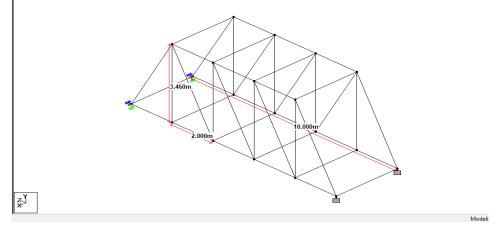
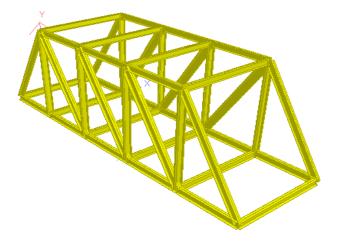


Fig 1. Geometry of structure



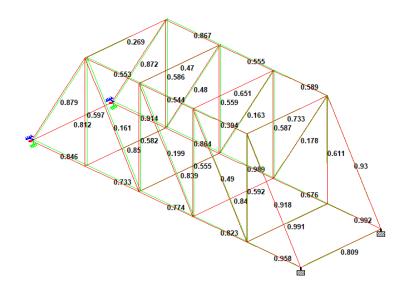
#### Fig.2. 3D Rendered View of Structure

## **Results and Discussion**

This chapter generally represents the results of analysis of foot over bridge for different bay spacings. Analysis has been done by using Staad Pro V8i software.

## a. Utility Ratio:

Utilization ratio for all the members is less than 1 for foot over bridge of 6m span and 2m bay spacing.



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Fig. 3 Utility Ratio for 10m span and 2m bay spacing

Load 7

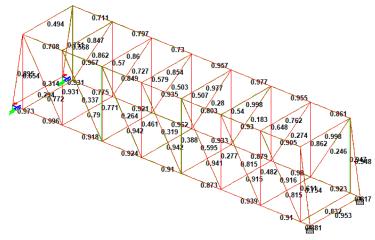


Fig. 4 Utility Ratio for 15m span and 2m bay spacing

#### b. Deflection :-

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Maximum Deflection for Different Span and Bay Spacing Arrangement is shown in table below –

| Sr. No. | Span Arrangement        | Maximum Deflection<br>(mm) |
|---------|-------------------------|----------------------------|
| 1       | 2m Bay Spacing 10m Span | 2.6252                     |
| 2       | 3m Bay Spacing 10m Span | 2.557                      |
| 3       | 4m Bay Spacing 10m Span | 3.733                      |
| 4       | 5m Bay Spacing 10m Span | 2.525                      |
| 5       | 2m Bay Spacing 15m Span | 5.272                      |
| 6       | 3m Bay Spacing 15m Span | 4.556                      |
| 7       | 4m Bay Spacing 15m Span | 5.393                      |
| 8       | 5m Bay Spacing 15m Span | 48.672                     |

#### c. Weight of the structure :-

i. for 10m span and 2m bay spacing

|    | PROFILE  | LENGTH (METE) | WEIGHT (KN |
|----|----------|---------------|------------|
| ST | ISLB175  | 36.68         | 6.002      |
| ST | ISLB75   | 4.00          | 0.237      |
| ST | ISLB100  | 10.00         | 0.784      |
| ST | ISLB125  | 30.00         | 3.480      |
| ST | ISWB150  | 3.00          | 0.500      |
| ST | ISSC100  | 39.96         | 7.829      |
| ST | ISLB150  | 2.00          | 0.278      |
| ST | ISLBP175 | 2.00          | 0.326      |
| ST | ISJB200  | 2.00          | 0.194      |
|    |          |               |            |
|    |          | TOTAL =       | 19.628     |

Fig. No. 5 Steel Take-off

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## ii. for 15m span and 2m bay spacing

|               | PROFILE  |  | LEP | GTH (METE) | wi wi | SIGHT (KN | ) |
|---------------|----------|--|-----|------------|-------|-----------|---|
|               |          |  |     |            |       |           |   |
| ST            | ISHB250  |  |     | 3.50       |       | 1.748     |   |
| $\mathbf{sr}$ | ISHB200  |  |     | 7.00       |       | 2.554     |   |
| ST            | ISSC140  |  |     | 4.00       |       | 1.303     |   |
| $\mathbf{sr}$ | ISHB150  |  |     | 10.00      |       | 2.650     |   |
| 3T            | ISSC120  |  |     | 17.99      |       | 4.617     |   |
| зт            | ISWB175  |  |     | 11.99      |       | 2.589     |   |
| 3T            | ISLB200  |  |     | 14.42      |       | 2.803     |   |
| 3T            | ISLBP175 |  |     | 2.00       |       | 0.326     |   |
| зT            | ISJB225  |  |     | 0.50       |       | 0.063     |   |
| 3T            | ISJB150  |  |     | 3.00       |       | 0.208     |   |
| зт            | ISHB225  |  |     | 3.50       |       | 1.476     |   |
| 3T            | ISSC150H |  |     | 3.00       |       | 1.085     |   |
| 3T            | ISLB150  |  |     | 7.00       |       | 0.973     |   |
| зт            | ISLB125  |  |     | 17.00      |       | 1.972     |   |
| 3T            | ISLB175  |  |     | 67.44      |       | 11.035    |   |
| эт            | ISMB175  |  |     | 3.00       |       | 0.574     |   |
| зт            | ISSC100  |  |     | 52.96      |       | 10.374    |   |
| 3T            | ISLB75   |  |     | 2.00       |       | 0.118     |   |
| зт            | ISWB150  |  |     | 6.99       |       | 1.166     |   |
|               |          |  |     |            |       |           |   |
|               |          |  |     | TOTAL      | =     | 47.632    |   |

| Sr.<br>No. | Span Arrangement           | Weight of str          | ucture (M.T.)         | Steel structure price per M.<br>T. |                       | %<br>Cost<br>saving |
|------------|----------------------------|------------------------|-----------------------|------------------------------------|-----------------------|---------------------|
|            |                            | Before<br>Optimization | After<br>Optimization | Before<br>Optimization             | After<br>Optimization |                     |
| 1          | 2m Bay Spacing 10m<br>Span | 3.86                   | 1.96                  | 231600                             | 117600                | 49.22               |
| 2          | 3m Bay Spacing 10m<br>Span | 4.27                   | 2.77                  | 256200                             | 166200                | 35.13               |
| 3          | 4m Bay Spacing 10m<br>Span | 3.65                   | 2.54                  | 219000                             | 152400                | 30.41               |
| 4          | 5m Bay Spacing 10m<br>Span | 3.08                   | 2.73                  | 184800                             | 163800                | 11.36               |
| 5          | 2m Bay Spacing 15m<br>Span | 7.28                   | 4.76                  | 436800                             | 285600                | 34.62               |
| 6          | 3m Bay Spacing 15m<br>Span | 5.49                   | 4.09                  | 329400                             | 245400                | 25.50               |
| 7          | 4m Bay Spacing 15m<br>Span | 4.99                   | 3.98                  | 299400                             | 238800                | 20.24               |
| 8          | 5m Bay Spacing 15m<br>Span | 6.15                   | 3.27                  | 369000                             | 196200                | 46.83               |

## Fig. No. 5 Steel Take-off

## d. Structure before optimization and after optimization: -

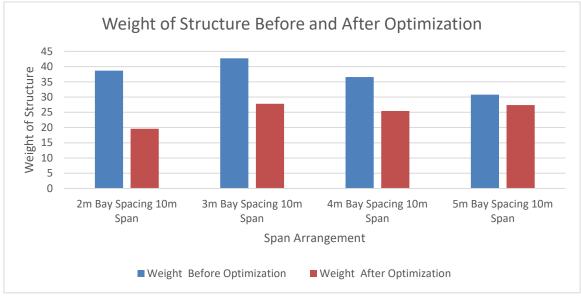


Fig. No. 8 Comparison Weight of Structure Before and After Optimization (10m Span)

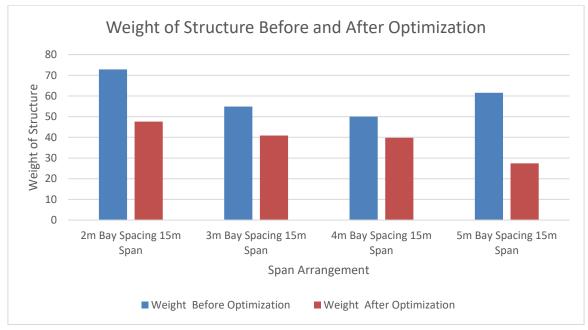


Fig. No. 8 Comparison Weight of Structure Before and After Optimization (15m Span)

## e. Comparison of maximum utility ratio: -

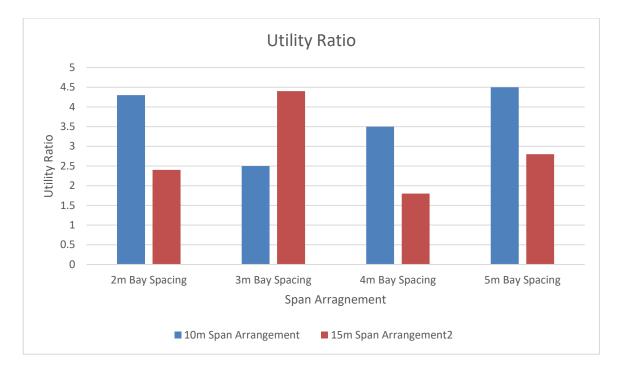


Fig. No. 9 Maximum utility ratio

f. Comparison of Deflection of Structure in mm: -

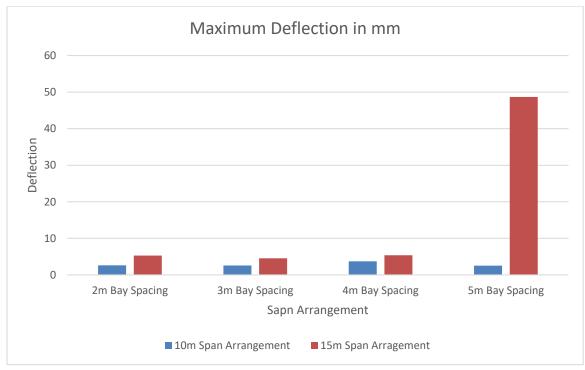


Fig. No.10 Comparison of Deflection of Structure in mm

## Conclusion

After analysis and design of steel foot over bridge by using STAAD PRO for different bay spacing and different span, the following points are observed

From software analysis, weight of steel foot over bridge after optimization is reduced by about 49% & 34% for 2m bay spacing for 10m and 15m span respectively i.e., economical section is achieved.

2m, 3m & 4m bay spacing steel foot over bridge are safe as maximum utility ratio is under 1.

The utility ratio increases with the increase in bay spacing for span under consideration.

We can say that 2m, 3m & 4m bay spacing can be comfortable with different span length, as structure is safe for these bay spacings.

The 5m bay spacing steel foot over bridge is not preferable as maximum utility ratio is above 1.

The economical steel foot over bridge after the analysis and design in 2m bay spacing and 10m span.

As the span of steel foot over bridge increases the weight of the steel foot over bridge will also be increased keeping the bay spacing constant.

## References

- [1] Punmia. B.C (2008) Design of steel structures Lakshmi Publications
- [2] Ramamurtham. S, Theory of structures, Dhanpat Publishing Company
- [3] A. Vashisth, M. Kumar, A. Kumar, Ashish, Design of Foot Bridge, 2012, doi: 10.13140/ RG.2.2.17428.81285
- [4] L. Muir, C.J. Duncan, The AISC 2010 Specification and the 14th Edition Steel Construction Manual, [in:] Structures Congress 2011, April 14–16, 2011, Las Vegas, Nevada, United States, pp. 661–675, doi: 10.1061/41171(401)58
- [5] B.R. Ellingwood, LRFD: implementing structural reliability in professional practice, Engineering Structures, 22(2): 106–115, 2000, doi: 10.1016/S0141-0296(98)00099-6
- [6] C.W. Roeder, Comparison of LRFD and allowable stress design methods for steel structures, 5th Seminario de Ingenieria Estructural, San Jose, Costa Rica, November, 1990
- S.-H. Lin, W. Yu, T.V. Galambos, ASCE LRFD method for stainless steel structures,
   [in:] CCFSS Proceedings of International Speciality Conference on Cold-Formed Steel
   Structures (1971–2018), 1, University of Missouri–Rolla, 1990,
   https://scholarsmine.mst.edu/ isccss/10iccfss/10iccfss-session5/1
- [8] T. Culp, R. Mathur, LRFD vs. ASD, Modern Steel Construction, 31(11): 24–27, 1991
- [9] R. Soegiarso, H. Adeli, Optimum load and resistance factor design of steel space-frame structures, Journal of Structural Engineering – ASCE, 123: 184–192, 1997, doi: 10.1061/(ASCE)0733-9445(1997)123:2(184)
- [10] M.S. Hayalioglu, S.O. Degertekin, Minimum cost design of steel frames with semi-rigid connections and column bases via genetic optimization, Computers & Structures, 83(21–22): 1849–1863, 2005 doi: 10.1016/j.compstruc.2005.02.009
- [11] IS 800 2007, General Construction in Steel code of Practice
- [12] Steel Table in S.I. Units

- [13] M.S. Hayalioglu, S.O. Degertekin, Minimum cost design of steel frames with semi-rigid connections and column bases via genetic optimization, Computers & Structures,83(21–22): 1849–1863, 2005 doi: 10.1016/j.compstruc.2005.02.009.
- [14] Dan Dubina (2008), 'Structural analysis and design assisted by testing of cold-formed steel structures,' Thin-Walled Structures, Vol 46, pp 741–764.
- [15] Chen, W.F. and Lian Duan. Bridge Engineering Handbook. New York: CRC Press, 1999.
  [5] Computers and Structures, Inc. CSI Analysis Reference Manual. Berkeley. CSI, 2008.
  [6] Crossroads". Ivy Tech Community College Fort Wayne. 2009.
- [16] S. Agrawal and M.K. Singha, "Numerical study on larg Span Pre-Stressed modified Warren Truss" vol. 4, pp. 320-325, 2015.
- [17] R. Ramasubramani, B. Sridhar, G. Arunkumar, and R. R. Bosco, "PLAN- NING, ANALYSIS AND DESIGN OF SELF SUSTAINABLE FOOT OVER BRIDGE WITH WIND TURBINE AND COMPARATIVE STUDY OF COMPOSITE AND STEEL MATERIAL," vol. 33, pp. 1274-1280, 2017.
- [18] P. S. S. B. Shubhank Gupta\*1, "COMPARATIVE ANALYSIS OF DIFFER- ENT TRUSS TYPE RAILWAY STEEL," vol. 6, po. 10, pp. 82-89, 2017.
- [19] V. A. Saluja and S. R. Satone, "Seismic Analysis of Foot Over Bridge for Different Soil Conditions," vol. 4, no. 3, pp. 352-356, 2016.
- [20] B. S. L. V. CHANDRIKKA1, "ANALYSIS AND DESIGN OF COLD FORMED STEEL FOOT OVER BRIDGE," pp. 2941-2946, 2019.
- [21] T. Prashanth and V. Gokulnath, "DESIGN AND ANALYSIS OF FOOT OVER," vol. 9, no. 1, pp. 639-651, 2018.
- [22] R. Wakte and V. Jeughale, "Comparative Seismic Analysis of Steel Foot Bridge for Human Resource Safety," pp. 53-57, 2018.
- [23] M. Kalpana and B. Mohabrao, "Analysis and design of foot bridge," vol. 119, no. 17, pp. 2875-2880, 2018.
- [24] B. Harbudiman, "Design of pedestrian truss bridge with Sengon-Rubber lam- inated veneer lumber Design of pedestrian truss bridge with Sengon-Rubber laminated veneer lumber," pp. 0-9, 2017.
- [25] S. Rajesh, "DESIGN OF A STEEL FOOT OVER BRIDGE IN A RAILWAYSTATION," vol. 8, no. 8, pp. 1533-1548, 2017
- [26] Durkee, Jackson L., "Foot Over Bridge Erected by Launching", Journal of The
- [27] Structural Division, ASCE, Vol. 98, No. ST7, Proc. Paper 9028, Pp. 1443-1463, July,1997
- [28] Durkee, Jackson, "Steel Bridge Construction", Bridge Engineering Handbook, Crcpress, PP 45-58, 2000.
- [29] Granath, P., "Distribution of Support Reaction Against A Steel Girder On A Launching Shoe." Journal of Constructional Steel Research, Vol. 47, No.3, Pp. 245-270, 1998.
- [30] G. Mylonakis and G. Gazetas. "Seismic soil-structure interaction: beneficial or detrimental", Journal of Earthquake Engineering, 4(03):277–301, 2000.
- [31] M.J.N. Priestley and R. Park. "Strength and ductility of concrete bridge columns under Seismic loading". ACI Structural Journal, 84(1):61–76, 1987.
- [32] D. Resendiz and J.M. Roesset. "Soil-structure interaction in Mexico City during the 1985

Earthquake. In Proc. of Int'l. Conf. on The Mexico Earthquakes – 1985", ASCE, volume Factors Involved and Lessons Learned, pages 193–203, 1987.

- [33] J.C. Wilson and B.S. Tan. "Bridge abutments: assessing their influence on earthquake response of meloland road overpass." Journal of Engineering Mechanics, 116(8):1838– 1856, 1990.
- [34] IS 11384:1985, "Code of practices for design of composite structure", Bureau of Indian Standards, New Delhi, India
- [35] Pise, Dr CP; Kadam, SS; Pawar, YP; Mohite, DD; Deshmukh, CM; Lakade, GD; "Performance-Based Seismic Design of Structures", International Journal Of Scientific Research, Vol. 4, Year- 2015.
- [36] Lakade, GD; Pise, Dr CP; Kadam, SS; Pawar, YP; Mohite, D; Deshmukh, CM; "Performance of RC Building under Dynamic Forces and Suitability of Strengthening by FRP Jacketing"; International Journal of Civil Engineering and Technology; vol. 6; 2015.
- [37] Sawant, VV; Pise, CP; Lakade, GD; "Experimental Study on High Performance Steel Fibre Reinforced Concrete using Metakaolin"; International Journal of Engineering Research & Technology (IJERT); Vol. 10, year 2021.